

BRIDGE SCOUR REPORT QUALITY CONTROL CHECKLIST

	PRO	JECT DETAILS	
Bridge A Route: Stream c County: Compan	rossing:		
QA Certi	fication:		
	C	HECKLISTS	
Checklists Completed: Hydrology Terrain HEC-RAS SRH 2D Env. Curves HEC-18	Designer(s):	Reviewer(s):	Date:
Instructions: 1. For all applicable spreadsheets, review 2. Originator shall make corrections as in			
3. Reviewer shall update status of resubr	nitted items, and provide addi	tional comments as needed.	
 If additional comments or corrections all items have a status of 4 (N/A) or 5 		make corrections and resubmit until	
 These checklists are intended to provid must be completed and included, alo 			icable checklists

Bri	dge Asset ID:	0			
Hy	drology QC Checklist		SCDOT Scour Critical Assessment and Management System		
initial date Originator: Technical Review By: QC Certified for Submittal:		 Popula Provide For each 	 instructions: 1. Populate "originator" & "review by" cells to left 2. Provide comments below per instructions on the Summary Sheet. 3. For each round of comment, add additional lines. 4. When all comments are satisfied, reviewer fills in date certified for submittal 		
п	QC Check and Description		Quality Control Review		
		Status*	QC Review Comment Originator Response		
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Bri	dge Asset ID: 0				
Hydrology QC Checklist		SCDOT Scour Critical Assessment and Management System			
initial date		 Provide For each 	ions: late "originator" & "review by" cells to left de comments below per instructions on the Summary Sheet. ach round of comment, add additional lines. In all comments are satisfied, reviewer fills in date certified for submittal		
ID	00 Charle and Description		Quality Control Review		
טו	QC Check and Description	Status*	* QC Review Comment Originator Response		
	General				
1	If a previously accepted model is used as the source for peak discharge(s),the source model is identified				
2	If peak discharge(s) are from a previously accepted model, discharges used agree with the source				
3	If 0.2% AEP discharge is extrapolated from 1% AEP discharge, confirm correct methodology				
	StreamStats				
4	Basin delineation				
5	Confirm rural vs. urban regression scenario				
<u> </u>	Basin characteristics				
7	Peak-flow report appears reasonable				
8	Unsteady Flow Hydrographs Source of stillwater height appropriate				
9	Development of hydrograph in accordance with SCDOT 2009 HDM				
10	Duration of time series extends past recession of storm surge				
11	Timing of storm surge plus tide represents worst case condition				
12					
13					
14					

agement System
Originator Response
Originator Response

Brio	Bridge Asset ID: 0				
HEC-RAS 1D QC Checklist initial date Originator: Technical Review By: QC Certified for Submittal:		SCDOT Scour Critical Assessment and Management System instructions: 1. Populate "originator" & "review by" cells to left 2. Provide comments below per instructions on the Summary Sheet. 3. For each round of comments, add additional lines. 4. When all comments are satisfied, reviewer fills in date certified for submittal			
ID	QC Check and Description	Status*	QC Review Comment Originator Response		
	General				
	Latest HEC-RAS version				
2	Free of error messages and warnings are justified				
3	Ground profile is reasonable and WSEL profile is close to parallel to ground.				
4	No hydraulic jumps				
5	Water surface elevations do not decrease going upstream, profile looks reasonable, WSEL drops through the structures are not excessive.				
	Cross-section Geometry				
6	All data on same datum				
7	Streamline follows channel and is placed upstream to downstream.				
8	Cross sections are perpendicular to direction of flow and do not intersect.				
9	Cross sections are not overtopped.				
10	Cross section data is entered from left to right (looking downstream)				
11	A skew coefficient is applied to any cross sections that intersect the stream at an angle greater than 20 degrees.				
12	Manning's n values				
13	Bank stations				
14	Structure-related cross sections 2 and 3 are located outside of the roadway fill and are parallel to the top of road.				
15	Appropriate contraction/expansion coefficients				
16	Ineffective flow and blocked obstruction areas are properly defined				
	Bridge Geometry				
17	Deck geometry is represented correctly				
	Abutment geometry represented correctly				
	Piers represented correctly				
20	Modeling approach is appropriate				
	Flow data				
21	All profiles (design storm, 1%,0.2%) and any additional profiles scoped as part of study are included in analyses.				

22	Starting boundary condition is appropriate and reasonable and the model incorporates a reasonable number of cross-sections between the downstream boundary condition and the area of analysis.		
23	All flow changes are appropriate		

	Boundary and Initial Conditions		
16	Boundary types & conditions established, documented, and reasonable		
17	Boundary locations reasonable & documented		
18	Flow distribution along Boundary Condition lines		
	If a time series data such as a discharge/stage hydrograph or a		
19	rating curve are assigned as a boundary condition, references are provided.		
20	Cold (dry) start or warm (wet) start simulation		
	Computation Setup		
21	Selected equation set is appropriate/justified		
22	Simulation period agrees with the boundary conditions		
23	Flow regime selected appropriately (e.g., subcritical flow or mixed flow regime)		
24	Computation Time Step is appropriate		
25	Model output locations and time intervals		
	Output Check		
26	Inundation extents stopped by the computation mesh boundary		
27	Note volume continuity (Generally, this is less than 1%.)		
28	Warning messages are acceptable		
29	No unstable computation results (like numerical surges of output hydrographs, flow depth, flow velocity, and water surface, etc.)		
30	If a steady state model is prepared, are the discharges constant?		
31	Sensitivity analysis for inundation extents and discharges: roughness coefficient, computation time step, etc.		
32	Initial conditions and final conditions are reasonable		
33	Model simulation is long enough to pass the entire hydrograph(s) through the model		
34	Are the maximum velocities reasonable and representative of peak flow conditions?		
35	Check Inundation Area		
36	Calibration & Validation / Reasonableness		
37	No lower recurrence interval WSE is greater than a higher recurrence interval WSE at the same section.		
	Result Summary/Submittal		
38	Summary of the model result tables matches hydraulic model output, such as maximum water surfaces, peak discharges, flow velocities, etc.		

Brie	Bridge Asset ID: 0					
Env	velope Curve Scour QC Checklist	SCDOT Scour Critical Assessment and Management System				
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ID	OC Check and Departmention		Quality Control Review			
	QC Check and Description	Status*	QC Review Comment Originator Response			
	Site Information					
1	Physiographic region					
2	LEW & REW stations at approach cross-section					
3	Unconstricted approach cross-section topwidth					
4	LEW & REW stations at bridge cross-section					
5	Left and right abutment toe stations at bridge					
6	Left and right top of bank stations at bridge					
	Channel topwidth					
8	Distance from toe to toe					
9	Left and right embankment lengths					
10	Geometric contraction ratio (m)					
11	WSEL used					
12	Left and right overbank widths					
	Clear-Water Abutment Scour Estimate					
13	Geometric-contraction ratio (m) range check					
14	Left embankment length range check					
15	Right embankment length range check					
16	Abutment scour depth (left)					
17	Abutment scour depth (right)					
18	Abutment scour hole topwidth curve (left)					
19	Abutment scour hole topwidth curve (right)					
	Abutment scour hole topwidth (left)					
21	Abutment scour hole topwidth (right)					
	Clear-Water Contraction Scour Estimate					
	Geometric contraction ratio(m) range check					
	Clear water contraction scour depth (left)					
24	Clear water contraction scour depth (right)					
	Live-Bed Contraction Scour Estimate					
	Geometric contraction ratio(m) range checks					
26	Live-bed contraction scour depth					
	Pier Scour Estimate					
	Pier locations					
	Pier or bent type					
	Pier widths					
	Pier lengths					
31	Pier angle of attack					

32	Estimate of minimum spacing		
33	Is final selected pier scour depth reasonable?		

	Pier scour		
	Pier shape		
29	Pier width and length		
30	Angle of attack		
31	Channel bed condition		
	Confirm flow depth upstream of pier		
33	Confirm flow velocity upstream of pier		
	D50 and D84 of bed material (for complex piers)		
35	Thickness of pile cap or footing (for complex piers)		
36	Height of pile cap or footing above bed before scour (complex piers)		
37	Distance from front of pile cap or footing to pier stem (complex piers)		
38	Number of columns per bent		
39	Pier spacing		
	Pier scour in cohesive bed materials		
	Identify bed material		
41	Correct equations used		
	Pier scour in coarse bed materials		
42	Correct equations used		